

SR 400 (I-4) Project Development and Environment (PD&E) Study FM No.: 432100-1-22-01

Pavement Type Selection Report





July 18, 2014

HNTB Corporation 610 Crescent Executive Court Suite 400 Lake Mary, FL 32746



Pavement Type Selection Report

SR 400 (I-4) Project Development and Environment (PD&E) Study

Segment 4: East of US 17/92 to East of SR 472

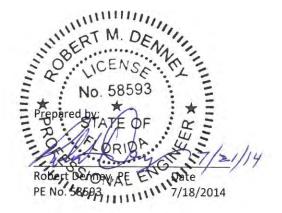
Volusia County, Florida

Contract Number: Financial ID Number: 432100-1-22-01 Federal Aid Project Number: 0041 227 1

Prepared For Florida Department of Transportation District 5 DeLand, Florida



July 18, 2014



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1.0 INTRODUCTION

The Florida Department of Transportation (FDOT) is proposing to reconstruct and widen I-4 as part of the I-4 Ultimate concept. This involves the build-out of I-4 to its ultimate condition through Central Florida, including segments in Polk, Osceola, Orange, Seminole and Volusia Counties. The concept design proposes the addition of two new express lanes in each direction within the center median of I-4, resulting in the reconstruction of the existing six-lane divided urban interstate to a ten-lane divided highway. The roadway improvements also include reconstruction of 19 local service interchanges and three systems interchanges.

The SR 400 (I-4) Project Development and Environment (PD&E) Study is a reevaluation project which addresses the revision from the original design concept showing two High Occupancy Vehicle (HOV) lanes, as recommended in the Environmental Impact Statement (EIS) for I-4 from SR 528 to SR 472, to the current proposed design concept of four Express Lanes. The Express Lanes are tolled lanes and will extend the full length of the project. The proposed typical section will include three general use lanes, two express lanes, an auxiliary lane (in some areas) and shoulders in each direction, with provision for a 44' rail corridor in the center median from US 27 to SR 528. The express lanes and general use lanes will be separated by two 10- or 12- foot shoulders with a barrier wall in between the shoulders.

The overall SR 400 (I-4) PD&E project limits include a total of approximately 41 miles of roadway improvements divided into two sections east and west of the I-4 Ultimate project. The approximate limits of improvement for the west section are from US 27 in Polk County to west of SR 435 (Kirkman Road) in Orange County and for the east section, from east of SR 434 in Seminole County to east of SR 472 in Volusia County. For purposes of documentation of the SR 400 (I-4) PD&E study, the east and west sections are further subdivided into segments as shown in Table 1.

SR 400 (I-4) F	PD&E West Section
Segment 1	CR 532 (Osceola/Polk County Line) to W. of SR 528 (Beachline Expressway) in
Segment 1	Osceola and Orange Counties (13.5 miles)
Sogmont 2	W. of SR 528 (Beachline Expressway) to W. of SR 435 (Kirkman Road) in Orange
Segment 2	County (3.6 miles)
Segment 5	US 27 to CR 532 (Osceola/Polk County Line) in Polk County (3.2 miles)
SR 400 (I-4) F	PD&E East Section
Segment 3	E. of SR 434 to E. of US 17/92 in Seminole County (10.2 miles)
Segment 4	E. of US 17/92 to E. of SR 472 in Volusia County (10.1 miles)
Ŭ	

Table 1: SR 400 (I-4) PD&E Segment Limits

The majority of the proposed improvements (37.4 miles) are within District 5 and a small segment (3.2 miles) is within District 1. The entire corridor is part of the state's Strategic Intermodal System (SIS).

As part of the SR 400 (I-4) PD&E Study, HNTB has prepared this Pavement Type Selection Report for I-4, Segment 4 (East of US 17/92 to East of SR 472) in Volusia County; a project location map is provided in Figure 1. The purpose of this report is to analyze, compare and select the most feasible pavement type for this project, utilizing the methods of the 1993 American Association of State Highway and Transportation Officials (AASHTO) Guide for Design of Pavement Structures, adopted by FDOT and described in detail in the FDOT Pavement Type Selection Manual (October, 2013).

2.0 PRINCIPAL FACTORS

2.1 Traffic

Pavement design for new alignment and reconstruction projects requires a structural loading forecast of the 18-KIP Equivalent Single Axle Load (ESAL). The accumulated 18-KIP ESALs are used to determine the Structural Number Required (SN_R) for flexible pavement and the Depth Required (D) for rigid pavement. While the total traffic volume is the main factor in determining roadway geometrics, the percent of commercial traffic and heavy load applications are the major influences in the structural pavement design. The I-4, Segment 4 corridor within the project area is expected to be utilized by local traffic and through traffic. To determine the ESALs for this project, traffic data was obtained from *the I-4 SAMR Update: Design Traffic Technical Memorandum (January, 2013).* Based on this memo, truck traffic percentages for the Segment 4 corridor range from 7.70 to 11.00 for year 2011. The truck factors for 2011 were reviewed for consistency by evaluating historical data provided by the FDOT Florida Traffic Online database. Based on these considerations, this project utilizes anticipated 24-hour truck traffic of 11.00% and a 20-year design. The future traffic volume projections used in the analysis are summarized in Table 2.

	Year	AADT
Opening Year	2020	115,000
Mid-Design Year	2030	128,800
Design Year	2040	142,700

Table 2	•	Future	Traffic	Projections
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The 18-KIP ESAL for the roadway is 24,720,000 for flexible pavement and 34,877,000 for rigid pavement. Based on this information, either asphaltic concrete (AC) or Portland cement concrete (PCC) pavement would be sufficient. Traffic information and ESAL calculations are provided in Appendix A.

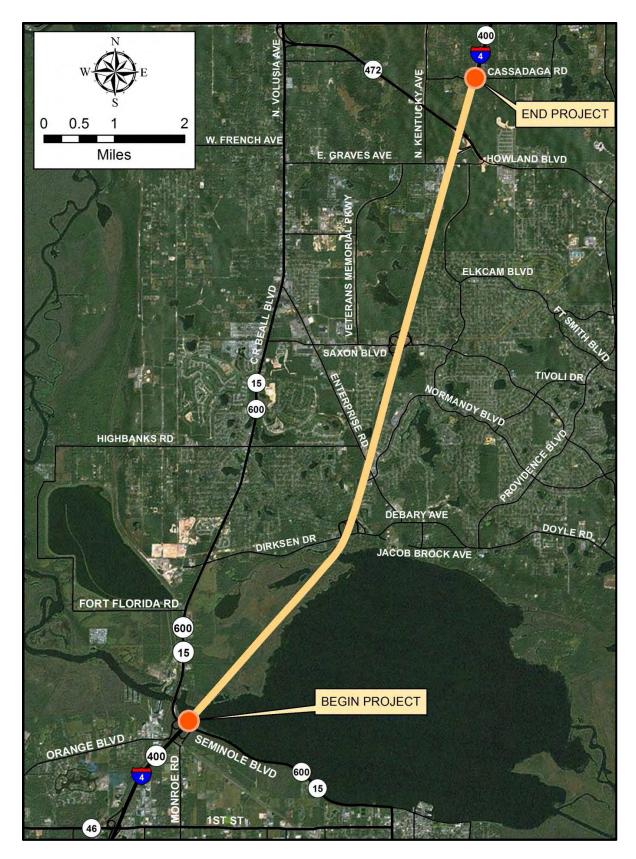


Figure 1: Project Location Map

2.2 Soil Characteristics

Geotechnical data near the study area was available from the I-4 (SR 400) widening and rehabilitation project, FPID: 408463-1-52-01. This project is located immediately north of the I-4, Segment 4 project and included pavement design data and calculations for the I-4/SR 400 mainline. The design value for the Resilient Modulus (M_R), which indicates the stiffness or strength of the roadbed soil, was 9000 psi for the widening portion of the project. This M_R value was used in preparing the PTSR for the I-4, Segment 4 project. The pavement design summary sheet from the S.R. 400 (I-4), FPID: 408463-1-52-01 project is included in Appendix B.

2.3 Weather

High rainfall intensities are experienced in Florida during portions of the year. These rainfall conditions are expected to equally affect subsoil conditions for both flexible and rigid pavements; thus, the weather does not favor the placement of one type of pavement over the other. Additionally, cross slopes are designed to drain water off the pavement, and drainable base and edge drains were considered in the economic analysis to ensure the runoff would not negatively impact the concrete pavement. Therefore, either AC or PCC pavement type could be constructed with satisfactory wet weather performance and durability.

2.4 **Construction Considerations**

The interstate will be completely reconstructed. Staged construction will be necessary for either rigid or flexible type of pavement. The available right-of-way will allow for either type to be constructed satisfactorily.

2.5 **Recycling**

The existing roadway pavement is to be completely reconstructed; therefore, there is an opportunity to recycle the existing asphalt pavement in the initial construction. FDOT has successfully recycled rigid and flexible pavement, therefore, there are future recycling opportunities for both pavement types during rehabilitation of the pavements.

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3.0 ECONOMIC ANALYSIS

The present worth method will be used to evaluate the cost of flexible pavement versus rigid pavement. All capital outlays for each alternative, including rehabilitation costs, are converted into today's dollars to compare the alternatives.

3.1 Basis of Comparison

The analysis will be based on the following assumptions: Analysis Period: 40 years Initial Pavement Design Life: 20 years Discount Rate: 3.5%

The following baseline rehabilitation strategies were considered, as recommended in the *Pavement Type Selection Manual (October 2013)* for concrete pavement and from supporting data for lifecycles of asphalt pavement in Volusia County:

Concrete Pavement – Limited Access (Mainline & Shoulder)

23 Year – Concrete Pavement Rehabilitation (3% Slab Replacement)* 33 Year – Concrete Pavement Rehabilitation (5% Slab Replacement)* *Estimate is based on the percentage of slab area in the truck lane

Asphalt Pavement - Limited Access (Mainline & Shoulder)

14 Year – Mill 3 inches 4" Structural Asphaltic Concrete
28 Year – Mill 3 inches 4" Structural Asphaltic Concrete

3.2 Pavement Data

The initial pavement designs developed for this analysis for both rigid and flexible pavement were based on the following geometry:

of Lanes=10 (3 GUL+2 SUL in each direction)
Lane Width=12 feet
GUL: Inside Shoulder Width=12 feet, Outside Shoulder Width=12 feet
SUL: Inside Shoulder Width=6 feet, Outside Shoulder Width=10 feet
Note: GUL = general use lane, SUL = special use lane

The typical section used for this analysis is provided in Appendix C and the pavement design calculations are provided in Appendix D.

Rigid Pavement - This pavement design has been prepared in accordance with the most recent Rigid Pavement Design Manual (RPDM) (FDOT Document No. 625-010-006-e, January, 2009). This

project is located in Volusia County. Using the Mechanistic-Empirical Pavement Design Guide (MEPDG) Design Tables, the slab thickness should be 12".

<u>Rigid Pavement Design Parameters</u> 18-KIP ESAL=34,877,000 Modulus of Subgrade Reaction (K_G)=200 pci Reliability (%R)=90%

Mainline 12" Concrete Depth 4" Optional Base Group 1 (Type B-12.5 Only) 12" Type B Stabilization

<u>Shoulder</u> 1.5" Type SP Structural Course (Traffic B) Optional Base Group 8 (9.5" LBR 100) 12" Type B Stabilization

Asphalt Pavement - This pavement design has been prepared in accordance with the most recent Flexible Pavement Design Manual (FPDM) (FDOT Document No. 625-010-002-g, March, 2008).

<u>Flexible Pavement Design Parameters</u> 18-KIP ESAL=24,720,000 (Traffic Level D) 18-KIP ESAL for shoulders=3% of mainline=741,600 (Traffic Level B) Resilient Modulus (M_R)=9,000 psi Reliability (%R)=90%

<u>Mainline</u>

 $SN_R = 5.31$ 0.75" Friction Course FC-5 (PG76-22) (Not included in the Life Cycle Cost Analysis) 2" Type SP Structural Course (Traffic D) (PG76-22) 3" Type SP Structural Course (Traffic D) Optional Base Group 12 (12.5" Limerock, LBR 100) 12" Type B Stabilization $SN_C = 5.41$

<u>Shoulder</u>

SN_R=3.08 1.5" Type SP Structural Course (Traffic B) Optional Base Group 8 (9.5" LBR 100) 12" Type B Stabilization SN_c=3.33

3.3 Cost Data for Economic Analysis

The unit prices used for this economic analysis are weighted averages obtained from FDOT's statewide item average unit costs from 03/01/2013 to 02/28/2014, current 12-month moving area averages for Area 6 (Alachua, Marion and Volusia Counties) and/or from D5 estimates provided by FDOT. The unit costs used are provided in Appendix E and are summarized in Table 3.

ltem	Price	Unit
Type B Stabilized (LBR 40)	\$3.25	Sq. Yd
OBG-1, Type B-12.5	\$9.42	Sq. Yd
OBG-8	\$14.64	Sq. Yd
OBG-12	\$15.00	Sq. Yd
Milling 1" Avg. Depth	\$2.24	Sq. Yd
Milling 3" Avg. Depth	\$1.17	Sq. Yd
Type SP Traffic Level B	\$85.00	Ton
Type SP Traffic Level D	\$85.00	Ton
Type SP Traffic Level D PG76-22	\$92.00	Ton
JPCP	\$55.00	Sq. Yd
CPR - Slab Replacement (3%)	\$400.00	Cu. Yd
CPR - Slab Replacement (5%)	\$400.00	Cu. Yd
Edgedrain (Draincrete)	\$26.72	Ft
Edgedrain Outlet Pipe (4 in)	\$30.67	Ft
Source: FDOT 12 month moving statewide averages, 2 and FDOT - D5 estimates.	L2-month moving area	averages for Area 6

 Table 3: Pavement Unit Prices

3.4 **Cost Comparison**

A life cycle economic analysis per mile of concrete pavement and asphalt pavement was performed using an analysis period of 40 years and a discount rate of 3.5%. Based on the life cycle cost analysis, the total present worth costs for concrete pavement is \$6,935,327 and for flexible pavement, \$6,034,332. The results of the analysis are summarized in Table 4. The details of the analysis are included in Appendix E.

4.0 SECONDARY FACTORS

4.1 **Performance of similar pavements in the area**

The existing pavement sections, west and east of the I-4 Segment 4 section are both constructed with AC pavement. In general, these sections have not experienced any areas of premature distress and maintenance resurfacing is not excessively disruptive. The average age to

rehabilitation for flexible pavements in Volusia County was reviewed. The average age to rehabilitation over the last 8 years in Volusia County ranged from 11.4 years to 17.5 years. In addition, the average age to rehabilitation for flexible pavements in Seminole County was also reviewed. The average age to rehabilitation over the 8-year period ending in 2013 for Seminole County ranged from 12.8 years to 22.1 years.

		Table 4	: Pavement Ty	pe S	Selection Economic Analy	vsis	
Con	crete Pa	vement	(PCC)				
			<u>Cost</u>		<u>P / F</u>		<u>PRESENT</u> <u>WORTH</u>
	Initial	2020	\$6,413,778	*	<u>1.00000</u>	=	\$6,413,778
23	Year	2043	\$590,219	*	<u>0.45329</u>	=	\$267,538
33	Year	2053	\$790,468	*	<u>0.32134</u>	=	\$254,011
					TOTAL AGENCY COSTS	=	\$6,935,327
					USER COSTS	=	N/A
					SALVAGE VALUE	=	N/A
		٦	TOTAL PRESEN	τw	ORTH LIFE-CYCLE COSTS	=	\$6,935,327
۸۶۵	halt Dav	ement (/	AC)				
Дэр	nant rav	ement (/	<u>Cost</u>		<u>P / F</u>		<u>PRESENT</u> <u>WORTH</u>
	Initial	2020	\$4,130,309	*	<u>1.00000</u>	=	\$4,130,309
14	Year	2034	\$1,976,451	*	<u>0.61778</u>	=	\$1,221,015
28	Year	2048	\$1,976,451	*	<u>0.38165</u>	=	\$754,321
					TOTAL AGENCY COSTS	=	\$6,105,646
					USER COSTS	=	N/A
					SALVAGE VALUE	=	\$71,314
		-	TOTAL PRESEN	TW	ORTH LIFE-CYCLE COSTS	=	\$6,034,332

Performance of concrete pavement in Central Florida was also reviewed. In the Orlando area within Orange County, concrete pavement was originally constructed on I-4 through the downtown area. This concrete pavement section has been in service for approximately 50 years and has undergone two major rehabilitations. Other concrete pavement sections in the Central Florida region were reviewed, including the average age to rehabilitation for concrete pavement in Hillsborough County. This data showed that over a 3 year period between 2006 and 2008, the average age for the rehabilitation cycle for these pavements within Hillsborough County were 20 years, 25 years and 22 years. Pavement performance and rehabilitation data is provided in Appendix F.

4.2 Adjacent Existing Pavements

The existing roadway sections, adjacent to the I-4 Segment 4 section are both constructed with flexible pavements. In addition, recent widening and rehabilitation projects throughout the corridor have been constructed with flexible pavement. The I-4, Segment 3 section, immediately west of Segment 4, is currently being evaluated for pavement type selection as part of the SR 400 (I-4) PD&E study.

4.3 **Conservation of Materials and Energy**

There are no significant differences in the energy consumption used to produce, transport or construct either type of pavement.

4.4 Availability of Local Materials or Contractor Capabilities

Materials are available locally for both pavement types. However, the majority of contractors in the Central Florida region are more familiar with asphalt pavement, since it is more commonly used in roadway projects in the area. FDOT District 5 also has prequalified contractors that have experience placing concrete pavement on major projects. Neither of the pavement types uses materials that are particularly scarce in Central Florida.

4.5 **Traffic Safety**

Current FDOT design guidelines and specifications for both the AC pavement and PCC pavement alternatives provide similar characteristics for wearing course, delineation through pavement and shoulder contrast, reflectivity under highway lighting and the maintenance of a nonskid surface.

4.6 **Incorporation of Experimental Features**

There are no experimental features included in this pavement type selection report.

4.7 **Stimulation of Competition**

Stimulation of competition is encouraged to avoid monopoly situations and improve products and methods in the projection of paving products. However, neither pavement type currently indicates a distinct advantage to provide more stimulation of competition over the other.

4.8 Municipal Preference, Participating Local Government Preference, and Recognition of Local Industry

No preferences apparent for pavement type by FDOT, which will be maintaining and operating this roadway facility.

5.0 CONCLUSIONS AND RECOMMENDATIONS

Based on the preceding life cycle cost analysis and considering all other design factors evaluated in this report, AC pavement has a long term owner's cost advantage of 13%. Therefore, it is recommended that asphalt pavement be considered as the pavement type for the SR 400 (I-4) Segment 4 corridor.

APPENDICES

APPENDIX A

TRAFFIC INFORMATION

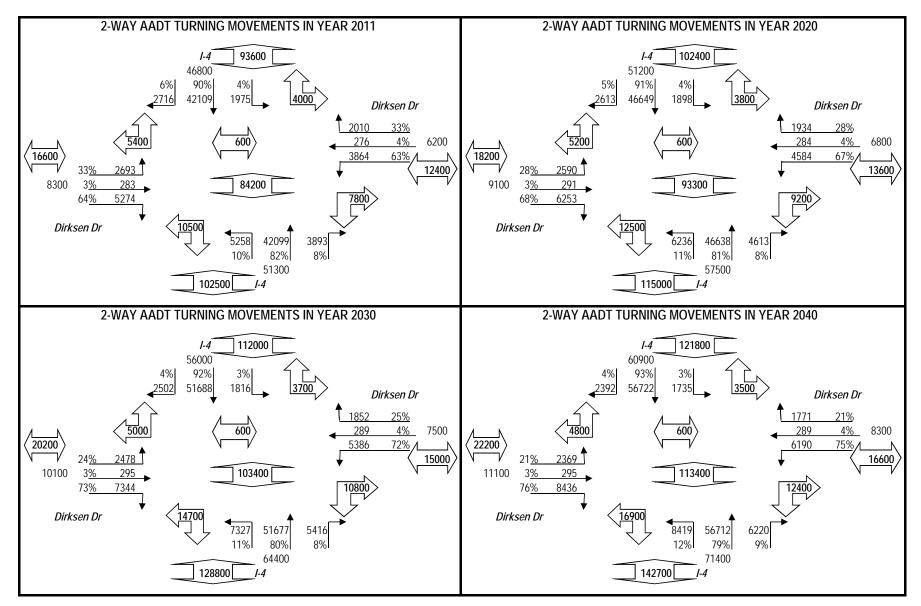
FLORIDA DEPARTMENT OF TRANSPORTATION TRANSPORTATION STATISTICS OFFICE 2012 HISTORICAL AADT REPORT

COUNTY: 79 - VOLUSIA

SITE: 0484 - ON I-4, 0.337 MI E. OF ST. JOHNS RIVER BRIDGE (ITS)

YEAR	AADT	DIRECTION 1	IRECTION 1 DIRECTION 2	*K FACTOR	D FACTOR	T FACTOR
2012	106500 C	E 53000	53000 W 53500	9.00	54.00	11.10
2011	102500 C	E 51500	51500 W 51000	9.00	54.30	11.00
2010	111500 C	E 56000	56000 W 55500	8.65	53.65	11.70
2009	107500 C	E 53500	53500 W 54000	8.67	54.57	11.70
2008	115000 T	E 58000	58000 W 57000	8.60	54.07	13.30
2007	118500 S	E 59500	59500 W 59000	8.30	56.39	10.80
2006	115500 F	E 58000	58000 W 57500	8.32	52.47	14.30
2005	111500 C	E 56000	56000 W 55500	8.40	52.60	7.10
2004	47000 C	E 25000	25000 W 22000	8.50	51.90	9.80
2001	84000 E	E 42000	42000 W 42000	11.20	56.30	7.60
2000	84000 C	E 42000	42000 W 42000	11.40	56.50	10.00
1999	65500 C	E 35000	35000 W 30500	11.00	57.00	18.40
1998	86000 C	E 43000	43000 W 43000	12.00	55.30	20.50
1997	74800 E	E 35200	35200 W 39600	11.90	53.90	7.30

AADT FLAGS: C = COMPUTED; E = MANUAL ESTIMATE; F = FIRST YEAR ESTIMATE S = SECOND YEAR ESTIMATE; T = THIRD YEAR ESTIMATE; X = UNKNOWN *K FACTOR: STARTING WITH YEAR 2011 IS STANDARDK, PRIOR YEARS ARE K30 VALUES



PROJECT TRAFFIC FOR I-4 AT Dirksen Dr: TO

PROJECT TRAF	FIC FOR	PD&E and DE	SIGN ANA	LYSIS INFO / FACTORS	
SECTION #: SEGMENT #: I TEM #: PROJECT DESCRIPTION: SI	ML	0	ve		
LOCATION DESCRIPTION:				LOCATION #: Mainline	1
GROWTH RATE FORMULA					
A: Interpolation B: Enter Growth Rate C: Enter All AADTs				here: C	1
D: New Facility		Line	ar Growth	Rate	%
""" order annicepotation failed of """ ""		Decayi	ng Growth (select	Rate	%
DESIGN INFORMATION		AADT	Daily	Direction Split	
Existing Year	2011	102500		(50% or 100%)	50%
Opening Year	2020	115000		Lanes in One Direction	3
Mid-Design Year Design Year	2030 2040	128800 142700		T24 values	
Design Teal	2040	142700		Existing to Opening Year Opening to Mid-Year	
				Mid-Year to Design-Year	
1995 EQUIVALENCY FACTORS	u(1)	1		inite Four to Boolgir Four	11.0070
(selected with an X)		FLEXIBLE PA	VEMENT	RIGID PAVEMEN	т
		SN = 5/THICK		SN = 12/THICK	
RURAL FREEWA		1.050	-	1.600	
URBAN FREEWA		0.900	<u> </u>	1.270	<u> </u>
RURAL HIGHWAY		0.960		1.350	
URBAN HIGHWA OTHER (Enter Factor a		0.890	<u> </u>	1.220	· · · · · · · · · · · · · · · · · · ·
OTTER (Enter Factor a	iiiu A).		—		
Equivalency Factors are based on Updated Pavement Dan ne Factors developed by Copes equation	mage Factors I		2, 1998		
ve reviewed the 18 kip Equivalent Single Axle DoadsqE the FDOT Project Traffic Forecasting Procedure wing	NO. 58	used for payement designed to the second sec		t. Thereby attest that these have been d Robert Denney, PE	leveloped in accordanc 4/7/2014
Org. Unit or Firm D	Petto.			Name	Date
Signature Mark Robinson, PE, Dis	ticks D	CENCIN'	-	EDOT DE	
Reviewed by: Name	11111	THAT		FDOT - D5 Org. Unit or Firm	Date

	YEARS SECTION #	5: 2011 to 204 #: 771190000 VEMENT URB SR 400 (I-4) - 1	0 SEGMENT #: SAN FREEWAY S. of Dirksen Drive	ML 0.900	N ANALYSIS ITEM #	INFO / FACTORS	
YEAR	AADT	ESAL (1000S)	ACCUM (1000s)	D	т	LF	EF
2011	102500	1015	0	0.5	11.00%	0.548	0.900
2012	103800	1025	0	0.5	11.00%	0.547	0.900
2013	105200	1037	0	0.5	11.00%	0.545	0.900
2014	106600	1049	0	0.5	11.00%	0.544	0.900
2015	108000	1061	0	0.5	11.00%	0.543	0.900
2016	109400	1072	0	0.5	11.00%	0.542	0.900
2017	110800	1084	0	0.5	11.00%	0.541	0.900
2018	112200	1095	0	0.5	11.00%	0.540	0.900
2019	113600	1107	0	0.5	11.00%	0.539	0.900
2020	115000	1118	1118	0.5	11.00%	0.538	0.900
2021	116300	1129	2247	0.5	11.00%	0.537	0.900
2022 2023	117700	1141	3388	0.5	11.00%	0.536	0.900
2023	119100 120500	1152	4540	0.5	11.00%	0.535	0.900
2024	120500	1164 1175	5704	0.5	11.00%	0.534	0.900
2025	123200	1186	6879 8065	0.5	11.00%	0.533	0.900
2020	123200	1197	9262	0.5		0.532	0.900
2028	126000	1208	10470	0.5	11.00% 11.00%	0.531 0.531	0.900
2029	127400	1200	11690	0.5	11.00%	0.530	0.900
2030	128800	1231	12921	0.5	11.00%	0.529	0.900
2031	130100	1241	14162	0.5	11.00%	0.528	0.900
2032	131500	1253	15415	0.5	11.00%	0.527	0.900
2033	132900	1264	16679	0.5	11.00%	0.526	0.900
2034	134300	1275	17954	0.5	11.00%	0.525	0.900
2035	135700	1286	19240	0.5	11.00%	0.524	0.900
2036	137100	1297	20537	0.5	11.00%	0.524	0.900
2037	138500	1308	21845	0.5	11.00%	0.523	0.900
2038	139900	1320	23165	0.5	11.00%	0.522	0.900
2039	141300	1331	24496	0.5	11.00%	0.521	0.900
2040	142700	1342	25838	0.5	11.00%	0.520	0.900
		,	MINIMUM				
		J.	Pper	ung to Resig	A Year ESAL	Accumulation (1000s): Accumulation (1000s):	11803 24720
have reviewed th		= ~	10 Crescont Exe	: *		ct. I hereby attest that these have le information.	
	Prepared by:		ake Mary, FL 32	140 . 4	-	Robert Denney, PE	4/7/2014
		Org. Unit-or F	STONAL	ENGIN		Name	Date
	Reviewed by:	Mark Robinso	on, PE//////	District 5 De	sian	FDOT - D5	
	Reviewed by.	Name		Title	sign	Org.Unit or Firm	Date

YEAR AADT (1000s) (1000s) D T LF EF 2011 102500 1431 0 0.5 11.00% 0.548 1270 2013 105200 1463 0 0.5 11.00% 0.545 1270 2014 106600 1480 0 0.5 11.00% 0.544 1270 2015 108000 1480 0 0.5 11.00% 0.544 1270 2016 108400 1513 0 0.5 11.00% 0.542 1270 2017 110800 1529 0 0.5 11.00% 0.540 1270 2020 11500 1578 0.5 11.00% 0.538 1270 2020 11500 1578 0.5 11.00% 0.538 1270 2021 11500 1578 0.5 11.00% 0.538 1270 2020 11500 1578 0.5 11.00% 0.531	SN=12	YEARS SECTION #	: 2011 to 204 : 771190000 /EMENT URE SR 400 (I-4) -		ML	N ANALYSIS ITEM #	INFO / FACTORS					
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2034 134300 1799 25331 0.5 11.00% 0.525 1.270 2035 135700 1815 27146 0.5 11.00% 0.524 1.270 2036 137100 1830 28976 0.5 11.00% 0.524 1.270 2037 138500 1846 30822 0.5 11.00% 0.523 1.270 2038 139900 1862 32684 0.5 11.00% 0.522 1.270 2039 141300 1878 34562 0.5 11.00% 0.520 1.270 2040 142700 1893 36455 0.5 11.00% 0.520 1.270 Coberling 40.01/d-Design Year ESAL Accumulation (1000s): 16653 Opening to Design Year ESAL Accumulation (1000s): 16653 Opening to Design Year ESAL Accumulation (1000s): 34877 Opening to Design Year ESAL Accumulation (1000s): 34877 Opening to Design Year ESAL Accumulation (1000s): 16653 <td cols<="" td=""><td>2032</td><td>131500</td><td>1767</td><td>21749</td><td>0.5</td><td>11.00%</td><td>0.527</td><td>1.270</td></td>	<td>2032</td> <td>131500</td> <td>1767</td> <td>21749</td> <td>0.5</td> <td>11.00%</td> <td>0.527</td> <td>1.270</td>	2032	131500	1767	21749	0.5	11.00%	0.527	1.270			
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2036 137100 1830 28976 0.5 11.00% 0.524 1.270 2037 138500 1846 30822 0.5 11.00% 0.523 1.270 2038 139900 1862 32684 0.5 11.00% 0.522 1.270 2039 141300 1878 34562 0.5 11.00% 0.521 1.270 2040 142700 1893 36455 0.5 11.00% 0.520 1.270 Coberting to Design Year ESAL Accumulation (1000s): 16653 Oberting to Design Year ESAL Accumulation (1000s): 16653 Oberting to Design Year ESAL Accumulation (1000s): 16653 Oberting to Design Year ESAL Accumulation (1000s): 34877 We reviewed the 18 kip Equivalent Single Axe Loads (E6AL St to be used for pavement design on this project. I hereby attest that these have been develop accordance with the FDOT Project Traffic Mecasting Produce Lesing historical traffic data and other available information. FDOT Project Traffic Mecasting Produce Lesing historical traffic data and other available information. ONAL ENT <	2034		1799	25331	0.5	11.00%	0.525	1.270				
2037 138500 1846 30822 0.5 11.00% 0.523 1.270 2038 139900 1862 32684 0.5 11.00% 0.522 1.270 2039 141300 1878 34562 0.5 11.00% 0.521 1.270 2040 142700 1893 36455 0.5 11.00% 0.520 1.270 Coberning to Design Year ESAL Accumulation (1000s): 16653 Oberning to Design Year ESAL Accumulation (1000s): 16653 Oberning to Design Year ESAL Accumulation (1000s): 34877 Oberning to Design Year ESAL Accumulation (1000s): 34877 <t< td=""><td></td><td></td><td>1815</td><td>27146</td><td>0.5</td><td>11.00%</td><td>0.524</td><td>1.270</td></t<>			1815	27146	0.5	11.00%	0.524	1.270				
2038 139900 1862 32684 0.5 11.00% 0.522 1.270 2039 141300 1878 34562 0.5 11.00% 0.521 1.270 2040 142700 1893 36455 0.5 11.00% 0.520 1.270 Oberhing to Abid Design Year ESAL Accumulation (1000s): 16653 Oberhing to Design Year ESAL Accumulation (1000s): 16653 Oberhing to Design Year ESAL Accumulation (1000s): 16653 Oberhing to Design Year ESAL Accumulation (1000s): 34877 Accumulation from the project Taffic Mecastric Procedure Ising historical traffic data and other available information. Fib Crescent Executive Ct. Suite 400 Org. Unit of Firm. Name Date Signature			1830	28976	0.5	11.00%	0.524	1.270				
2039 141300 1878 34562 0.5 11.00% 0.521 1.270 2040 142700 1893 36455 0.5 11.00% 0.520 1.270 Copening to Design Year ESAL Accumulation (1000s): 16653 Opening to Design Year ESAL Accumulation (1000s): 34877 Ner reviewed the 18 kip Equivalent Single Axte Loads (E6AL S) to be used for pavement design on this project. I hereby attest that these have been develop accordance with the FDOT Project Traffic Mecasting Probabure Using historical traffic data and other available information. 610 Crescent Executive Ct, Suite 400 Prepared by: HNTB Oke Mary, FL 32746 ONAL Reviewed by: Mark Robinson, P6 Signature ONAL E ONAL E Reviewed by: Mark Robinson, P6			1846	30822	0.5	11.00%		1.270				
2040 142700 1893 36455 0.5 11.00% 0.520 1.270 Interviewed the 18 kip Equivalent Single Axte Loads (EGAL 9) to be used for pavement design on this project. I hereby attest that these have been develop accordance with the FDOT Project Traffic Mecastro Protecture Using historical traffic data and other available information. Prepared by: HNTB Opening to Design Year ESAL Accumulation (1000s): 34877 Attendesign on this project. I hereby attest that these have been develop accordance with the FDOT Project Traffic Mecastro Protecture Using historical traffic data and other available information. Opening to Design Year ESAL Accumulation (1000s): 34877 Opening to Design Protecture Using historical traffic data and other available information. Opening to Design Year ESAL Accumulation (1000s): 34877 Opening to Design Year ESAL		139900	1862	201-11.001	0.5	11.00%	0.522	1.270				
Ve reviewed the 18 kip Equivalent Single Axe Loads (ESAL St to be used for pavement design on this project. I hereby attest that these have been develop accordance with the FDOT Project Traffic Mecasting Brocadure using historical traffic data and other available information. Prepared by: HNTB Take Mary, PL 32746 Robert Denney, PE 4/7/201 Org. Unit of Firm, Store Construction, Name Date Signature Signature Signature Signature To Design FDOT - D5					0.5	11.00%	0.521	1.270				
Ve reviewed the 18 kip Equivalent Single Axte Loads (EGAL s) to be used for pavement design on this project. I hereby attest that these have been develop accordance with the FDOT Project Traffic Mecasting Procedure using historical traffic data and other available information. Prepared by: HNTB 510 Crescent Executive Ct. Suite 400 Prepared by: HNTB 510 Crescent Executive Ct. Suite 400 Org. Unit of prime 247/201 Org. Unit of prime 247/201 Signature 500 CAL EV Reviewed by: Mark Robinson, PE	2040	142700	1893	36455	0.5	11.00%	0.520	1.270				
Ave reviewed the 18 kip Equivalent Single Axte Loads (ESAL s) to be used for pavement design on this project. I hereby attest that these have been develop accordance with the FDOT Project Traffic Mecasting Probabure Using historical traffic data and other available information. Prepared by: HNTB Fake Mary, PL 32746 Robert Denney, PE 4/7/201 Org. Unit of Difficient Content of State of S				C Popehing 4	oAMid-Desig	n Year ESAL	Accumulation (1000s):					
Org. Unit of Firm. Name Date Signature	ve reviewed the	accordance with	the FDOT Project	C (C C C C C C C C C C C C C C C C C C	AMid-Desig ing to Desig for pavement de dedure using hi	in Year ESAL	Accumulation (1000s): ct. I hereby attest that these have a and other available information.	16653 34877 been develope				
Signature SONAL EN Reviewed by: Mark Robinson, PETITION District 5 Design FDOT - D5		Prepared by:			46			4/7/201				
Reviewed by: Mark Robinson, Parining Strict's Design FDOT - D5			1	A St. C	A CHAN		Name	Date				
Reviewed by: Mark Robinson, Parining Strict's Design FDOT - D5			orginature	1, ONAL	ELUIN							
Name Title Org.Unit or Firm Date		Reviewed by:	Mark Robins	on, Permin	Platnict 5 De	sign						
			Name	1	itle		Org.Unit or Firm	Date				

APPENDIX B

GEOTECHNICAL INFORMATION

Florida Department of Transportation Flexible Pavement Design Summary Sheet

Prepared By:	Edward G. Wolfson, P.E.	Date Prepared:		7-Jan-0	5
Financial Project ID:	408463-1-52-01	U.S. No.:	I-4	S.R. No.:	400
FAP No.:	N/A	From:	East of SR	472	
Section No:	79110	To:	West of SR	44	
County:	Volusia	Begin Mp:	10.254		
Project Length:	17,900' (3.390 Mi)	End Mp:	13.644		
Type of Work:	Mill, Resurf. & Widening	%R:	95%		
Opening Year:	2010	M _r :	25,000 resu	irfacing / 9,000 wide	ening
Design Year:	2030	Design Speed:	70 MPH		
ESAL _D :	24,808,000	Design Seq No.:	N/A		
SN _R :	3.95 resurf / 5.59 widening	Project Name:	SR 400 from	n East of SR 472 to	West of SR 44

Existing SN:

4.32

S.R. 400 Mainline Existing Pavement: (Good Condition)

LAYER	THICKNESS (in.)	<u>COEFF</u>	<u>SN</u>
FC-2	1/2	0.00	0.00
Type S Asphaltic Concrete	3	0.34	1.02
Туре I	1	0.15	0.15
Binder	2	0.15	0.30
Limerock Base Course	10 1/2	0.18	1.89
Stabilized Subgrade	12	0.08	0.96

Recommended Widening Pavement Design:

LAYER	THICKNESS (in.)	COEFF	<u>SN</u>
FC-5 (PG 76-22)	3/4	0.00	0.00
Type SP Asphalt Concrete (Traffic Level D)*	6 1/2	0.44	2.86
OBG 9	10	0.18	1.80
Stabilized Subgrade (LBR 40)	12	0.08	0.96
		SN:	5.62

Recommended Milling and Resurfacing Design⁺:

LAYER	THICKNESS (in.)	<u>COEFF</u>	<u>SN</u>
Existing Pavement (See Above)			4.32
Milling (3-3/4" depth) ++	3 1/4	-0.34	-1.11
FC-5 (PG 76-22)	3/4	0.00	0.00
Type SP Asphalt Concrete (Traffic Level D)*	3	0.44	1.32
		SN:	4.54

* (PG 76-22) is to be used in the top structural course

* The milling and resurfacing for this project is intended for the purpose of facilitating the proper construction of the proposed widening and for the purpose of providing a consistent surface across the proposed typical section.

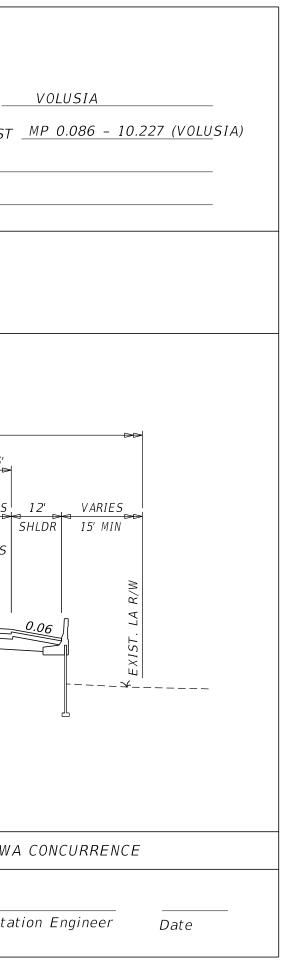
⁺⁺ The milling depth that is specified is 3-3/4" however, 1/2" of this depth is the existing FC-2 therefore only 3-1/4" of structural asphalt is being removed.

Approved by:	Concurrence by:	Concurrence by:	Concurrence by:
Rhet L. Schmidt, P.E.	Donald M. Barnhouse	Derek A. Fusco, P.E.	Annette K. Brennan, P.E.
Responsible Engineer	Dist Pavt Design Engineer	FHWA Area Engineer	Dist Design Engineer
Date:	Date:	Date:	Date:

APPENDIX C

TYPICAL SECTION

FINANCIAL PROJECT ID432100-1-22-01	FEDERAL AID PROJECT	T NO	N/A		COUNTY N	IAME _
SECTION NO79110					LIMITS/M	ILEPOST
PROJECT DESCRIPTIONI-4 WIDENING FROM EAST	01 03 11-32 10 LAST (<u> </u>				
PR	OPOSED ROADW	IAY TYI	PICAL S	ECTION		
VARIES (150' MIN)		₂ (į CONS	STRUCTION I-4	VARIES	(150' MIN)	
0'-36' 36' GUL	55'	⊲	<u>55'</u> DR		36' GUL	0'-36'
VARIES 12' VARIES 12' 12' 12' 12' 12' 12' 12' 12' 12' 12'	R SHLDR 12' 12'		12' 10' SHLDR	12' 12'	12' 12'	VARIES
$\begin{bmatrix} AUX\\LANES\\ I \\ I$		1 0.02	1	0.02 0.02	↑ ↑ 0.02 0.03	AUX LANES
	TYPICAL S SR 400 (INTE MP 0.086 TO 10.227 ((STA. 2583+00.00 TC DESIGN SPEE	RSTATE 4 VOLUSIA D STA. 311	COUNTY) 8+46.00)			
APPROVED BY:		ONCURREN				FHW
HNTB CORPORATION 610 CRESCENT EXECUTIVE CT.DBERT M. DENNEY, P.E. Date ngineer Of Record 58593LAKE MARY, FL 32746 (407) 805-0355 CERT OF AUTH NO 6500	ANNETTE K. BRENNAI FDOT District Design		Date	_	FHWA Tr	ansporta



APPENDIX D

PAVEMENT DESIGN CALCULATIONS

Pavement Design For New Pavement (Flexible)

Project:	SR 400 (I-4) Mainline														
Given:	Opening + 20 years = $ESAL_D = 24,720,000$ $M_R = 9,000 psi$ Assume a 90% reliability				Traffic Level		D								
1.0	From table 5.3, the Structural Number I	Require	ed (SN _R) =						5.31						
2.0		SN _R	=		SNc										
		5.31	=	a ₁	D ₁	+	a_2	D_2	+	a ₃	D_3	+	a_4	D_4	
		5.31	=	0	0.75	+	a_2	D_2	+	a ₃	D_3	+	0.08	12	
		5.31	=		0.00	+	a ₂	D_2	+	a ₃	D_3	+		0.96	
		4.35	=				a ₂	D_2	+	a ₃	D_3				
3.0	With the following eqn. find the base gr	oup fro	m table 5.9												
		4.35	=	a ₂	D_2	+	a ₃	D_3							
Base group	12		yields a		5.00		inch str	uctural c	ourse wit	h an SN of		4.45			
0 1	Note: the structural number found in ta	able 5.9	•	ly larg	er than the a	₂ D ₂ +	a ₃ D ₃ ratio								

4.0

Calculate the Structural number (SN_{c}), so that it is equal to or larger than SN_{R} .

Material	Thickness	Coefficient	SN _C	
Structural Course	5.00	0.44	2.20	see table 5.4
Base (OBG 12 - 12.5" Limerock - LBR 100)	12.50	0.18	2.25	see table 5.6
Stabilization (LBR 40)	12.00	0.08	0.96	
Total thickness	29.50 inches	s SN _C =	5.41	

 $\begin{array}{rrr} {\sf SN}_{\sf C} & \geq & {\sf SN}_{\sf R} \\ 5.41 & \geq & 5.31 \end{array}$

New Pavement Design (Modulus of Subgrade Reaction = 200) (Rigid)

 REQUIRED DEPTH (D_R) FOR 90% RELIABILITY
 From table 3.2

 ESAL's
 Depth

 35,000,000
 11.5"

 Table E.8 from the 2009 FDOT Rigid Pavement Design Manual -Based on MEPDG with Tied Concrete Shoulders

 When an ashphalt shoulder is used, Mainline Slab thickness must be increased by 1/2" and a 14' wide slab used.

Pavement Design For New Pavement (Flexible)

Project:	SR 400 (I-4) Opening Year 20		noulder										
	Design Year 204												
Given:	ESAL _D = 741,600	l .	٦	Fraffic Level		В							
	M _R = 9,000 psi												
	Assume a 90% re	eliability											
1.0	From table 5.3 (o	r A.4a), the Struc	tural Num	ber Required (S	N _R) =			3.08					
2.0	SN	. =		SNc									
	3.08	3 =	a ₁	D ₁	+	a ₂	D_2	+	a_3	D_3	+	a ₄	D_4
	3.08	3 =	0	0.75	+	a ₂	D_2	+	a_3	D_3	+	0.08	12
	3.08	3 =	0.00		+	a ₂	D_2	+	a_3	D_3	+	0.96	
	2.12	2 =				a ₂	D_2	+	a_3	D_3			
3.0	With the following	g eqn. find the ba	se group fr	rom table 5.9									
	2.12	2 =	a ₂	D_2	+	a ₃	D_3						
Base group	8	yields a	1.50	inch structural	course	with an S	N of				2.28		
	Note: the structu	ral number found	l in table 5.	.9 must be sligh	tly larger	than the	a ₂ D ₂ +	a ₃ D ₃ ratio)				

4.0 Calculate the Structural number (SN_C), so that it is equal to or larger than SN_R .

Material	Thickness	Coefficient	SN _C	
Structural Course (Traffic Level B)	1.50	0.44	0.66	see table 5.4
Base (OBG 8- LBR 100)	9.50	0.18	1.71	see table 5.6
Stabilization (LBR 40)	12.00	0.08	0.96	
		SN _C =	3.33	_

$$\begin{array}{rrrr} {\sf SN}_{\sf C} & \geq & {\sf SN}_{\sf R} \\ 3.33 & \geq & 3.08 \end{array}$$

APPENDIX E

LIFE CYCLE COST ANALYSIS

FLORIDA DEPARTMENT OF TRANSPORTATION

PAVEMENT TYPE SELECTION SPREADSHEET

PROJECT DESCRIPTION:

Financial Project ID:	432100-1-22-01
State Road Number:	SR 400
County:	Volusia
Project Length:	10 Miles
Roadway ID:	79110000
Begining MP:	
Ending MP:	
Transportation System:	
Type of Work	
Design Version	



	432100-1-22-01							
	LIST OF CONSTRUCTION ITEMS							
Pay Item	Description	Mean Price	St. Deviation	Unit				
160 4	Type B Stabilized (LBR 40)	\$3.25		Sq. Yd				
285 7	OBG-1, Type B-12.5	\$9.42		Sq. Yd				
285 7	OBG-8	\$14.64		Sq. Yd				
285 7	OBG-12	\$15.00		Sq. Yd				
327 70	Milling 1" Avg. Depth	\$2.24		Sq. Yd				
327 70	Milling 3" Avg. Depth	\$1.17		Sq. Yd				
334 1	Type SP Traffic Level B	\$85.00		Ton				
334 1	Type SP Traffic Level D	\$85.00		Ton				
334 1	Type SP Traffic Level D PG76-22	\$92.00		Ton				
350 1	JPCP	\$55.00		Sq. Yd				
353 70	CPR - Slab Replacement (3%)	\$400.00		Cu. Yd				
353 70	CPR - Slab Replacement (5%)	\$400.00		Cu. Yd				
446 1	Edgedrain (Draincrete)	\$26.72		Ft				
446 71	Edgedrain Outlet Pipe (4 in)	\$30.67		Ft				

LIFE CYCLE COST ANALYSIS JOINTED PLAIN CONCRETE PAVEMENT DESIGN (RIGID PAVEMENT) Financial Project ID:432100-1-22-01, SR No.-SR 400, County: Volusia Project Length: 10 Miles, Roadway ID: 79110000 Begining MP: , Ending MP: **Definitions:** 5280 40 Length of Section: Ft Analysis Period: Ft 3.5 Passing Lane Width: 12 Discount Rate: Travel Lane Width: Ft 2020 14 Initial Year of Construction: Inside Shoulder Width: 18 Ft No. of Passing Lanes: 3 Outside Shoulder Width: 18 Ft No. of Travel Lanes: 2 Total Pavement Area: 675,840 Sq. Ft No. of Travel Directions: 2 380,160 Sq. Ft 63,360 Long. Concrete Joints (Ft) 45,056 Total Shoulder Area: Trans. Concrete Joints (Ft) PRESENT CONSTRUCTION ITEMS ТНК. UNIT UNIT PRICE ST DEV QTY. COST WORTH INITIAL CONSTRUCTION IN YEAR: 0 MAINLINE: JPCP 12 75,093.3 \$0.00 \$4,130,133 \$4,130,133 Sq. Yd \$55.00 OBG-1, Type B-12.5 4 75,093.3 Sq. Yd \$9.42 \$0.00 \$707,379 \$707,379 Type B Stabilized (LBR 40) 12 75,093.3 Sq. Yd \$3.25 \$0.00 \$244,053 \$244,053 Edgedrain (Draincrete) 1 10,560.0 Ft \$26.72 \$0.00 \$282,163 \$282,163 Edgedrain Outlet Pipe (4 in) 50.0 Ft \$30.67 \$0.00 \$1,534 \$1,534 1 SHOULDER: Type SP Traffic Level B 1.5 3,445.2 Ton \$85.00 \$0.00 \$292,842 \$292,842 OBG-8 9.5 42,240.0 Sq. Yd \$14.64 \$0.00 \$618,394 \$618,394 Type B Stabilized (LBR 40) 12 42.240.0 Sq. Yd \$3.25 \$0.00 \$137,280 \$137.280 **DESIGN COSTS:** Subtotal MOT COSTS: Subtotal **CEI COSTS:** Subtotal **REHABILITATION IN YEAR:** 23 MAINLINE: CPR - Slab Replacement (3%) 12 750.9 \$400.00 \$300,373 Cu. Yd \$0.00 \$136,155 SHOULDER: Milling 1" Avg. Depth 42,240.0 Sq. Yd \$2.24 \$0.00 \$94,618 \$42,889 1 Type SP Traffic Level B 1 2,296.8 Ton \$85.00 \$0.00 \$195,228 \$88,494 **DESIGN COSTS:** Subtotal MOT COSTS: Subtotal

Subtotal

CEI COSTS:

LIFE CYCLE COST ANALYSIS

JOINTED PLAIN CONCRETE PAVEMENT DESIGN (RIGID PAVEMENT)

Financial Project ID:432100-1-22-01, SR No.-SR 400, County:Volusia Project Length: 10 Miles, Roadway ID: 79110000



Definitions:			Begining MP: , Ending MP:		
Length of Section:	5280	Ft		Analysis Period:	40
Passing Lane Width:	12	Ft		Discount Rate:	3.5
Travel Lane Width:	14	Ft		Initial Year of Construction:	2020
Inside Shoulder Width:	18	Ft		No. of Passing Lanes:	3
Outside Shoulder Width:	18	Ft		No. of Travel Lanes:	2
Total Pavement Area:	675,840	Sq. Ft		No. of Travel Directions:	2
Total Shoulder Area:	380,160	Sq. Ft	63,360 Long. Concrete Joints (Ft)	45,056 Trans. Concrete J	oints (Ft)

CONSTRUCTION ITEMS	ТНК.	QTY.	UNIT	UNIT PRICE	ST DEV	COST	PRESENT WORTH
REHABILITATION IN YEAR: MAINLINE:	33]					
CPR - Slab Replacement (5%)	12	1,251.6	Cu. Yd	\$400.00	\$0.00	\$500,622	\$160,871
SHOULDER:							
Milling 1" Avg. Depth Type SP Traffic Level B	1 1	42,240.0 2,296.8	Sq. Yd Ton	\$2.24 \$85.00	\$0.00 \$0.00	\$94,618 \$195,228	\$30,405 \$62,735
DESIGN COSTS: MOT COSTS: CEI COSTS:			Subtotal Subtotal Subtotal				
REHABILITATION IN YEAR: MAINLINE:	40]					
SHOULDER:							
DESIGN COSTS: MOT COSTS: CEI COSTS:			Subtotal Subtotal Subtotal				
REHABILITATION IN YEAR:]					
DE E/	TOTAL INITIA	L CONSTRU	JCTION CO	ST (YEAR 2020):			\$6,413,778
	TOTAL PRES	ENT WORT	H REHABII	LITATION COST:			\$521,549
A THE TOP TERMINO	ΤΟΤΑ	L PRESENT	WORTH SA	LVAGE VALUE:			\$0
			PR	ESENT WORTH:			\$6,935,327

LIFE CYCLE COST ANALYSIS ASPHALT CONCRETE PAVEMENT DESIGN (FLEXIBLE PAVEMENT) Financial Project ID:432100-1-22-01, SR No.-SR 400, County:Volusia



'inancial Project ID:432100-1-22-01, SR No.-SR 400, County:Volus Project Length: 10 Miles, Roadway ID: 79110000

Begining MP: , Ending MP:

Definitions:

Length of Section:	5280
Passing Lane Width:	12
Travel Lane Width:	12
Inside Shoulder Width:	18
Outside Shoulder Width:	22
Total Pavement Area:	633,600
Total Shoulder Area:	422,400

Ft Ft Ft

Ft Ft Sq. Ft Sq. Ft

Analysis Period:
Discount Rate:
Initial Year of Construction:
No. of Passing Lanes:
No. of Travel Lanes:
No. of Travel Directions:

40
3.5
2020
5
2

INITIAL CONSTRUCTION IN YEAR: 0 MAINLINE:	CONSTRUCTION ITEMS	ТНК.	QTY.	UNIT	UNIT PRICE	ST DEV	COST	PRESENT WORTH
Type SP Traffic Level D PG76-22 Type SP Traffic Level D 0BG-12 Type SP Stabilized (LBR 40) 2 12.5 12 7,656.0 11,484.0 70,400.0 Sq. Yd Ton SS, Vd S92.00 S0.00 S704,352 S0.00 S704,352 S704,352 S704,352 S704,352 SHOULDER: 12 7,656.0 70,400.0 Sq. Yd S3.25 S0.00 S325,380 S1.056,000 S228,800 SHOULDER: 12 3,828.0 46,933.3 Ton Sq. Yd S45.00 S0.00 S325,380 S687,104 S325,380 S687,104 DBG-8 0BG-8 0BG-8 0BG-8 0DG CoSTS: 9.5 S46,933.3 Sq. Yd S41.64 S0.000 S000 S325,380 S687,104 S487,104 S687,104 Type B Subilized (LBR 40) 12 Subtotal Subtotal Subtotal Subtotal Subtotal Subtotal S152,533 S152,538 S50.885 S152,533 PERIABILITATION IN YEAR: 14 14 S1.17 S0.00 S82,368 S50.885 S10,50 S50.885 S10,50 S10,51,31 S64,948 S10,028 SHOULDER: 1 1 14 S1.17 S0.00 S10,51,31 S64,948 S10,028 S60,760 S402,028 SHOULDER: 1 1 14 S1.00 Ton S85.00 S0.00 S105,131 S64,948 S100,028 S64,948 S10	INITIAL CONSTRUCTION IN YEAR:	0						
Type SP Traffic Level D PG76-22 Type SP Traffic Level D 0BG-12 Type SP Stabilized (LBR 40) 2 12.5 12 7,656.0 11,484.0 70,400.0 Sq. Yd Ton SS, Vd S92.00 S0.00 S704,352 S0.00 S704,352 S704,352 S704,352 S704,352 SHOULDER: 12 7,656.0 70,400.0 Sq. Yd S3.25 S0.00 S325,380 S1.056,000 S228,800 SHOULDER: 12 3,828.0 46,933.3 Ton Sq. Yd S45.00 S0.00 S325,380 S687,104 S325,380 S687,104 DBG-8 0BG-8 0BG-8 0BG-8 0DG CoSTS: 9.5 S46,933.3 Sq. Yd S41.64 S0.000 S000 S325,380 S687,104 S487,104 S687,104 Type B Subilized (LBR 40) 12 Subtotal Subtotal Subtotal Subtotal Subtotal Subtotal S152,533 S152,538 S50.885 S152,533 PERIABILITATION IN YEAR: 14 14 S1.17 S0.00 S82,368 S50.885 S10,50 S50.885 S10,50 S10,51,31 S64,948 S10,028 SHOULDER: 1 1 14 S1.17 S0.00 S10,51,31 S64,948 S10,028 S60,760 S402,028 SHOULDER: 1 1 14 S1.00 Ton S85.00 S0.00 S105,131 S64,948 S100,028 S64,948 S10			-					
Type SP Traffic Level D 3 11,484.0 Ton \$85.00 \$0.00 \$976,140 \$976,140 OBG-12 12,5 70,400.0 \$6, Yd \$15.00 \$0.00 \$1,056,000 \$1,056,000 \$1,056,000 \$228,800 \$228,800 SHOULDER: 3 3828.0 Ton \$85.00 \$0.00 \$325,380 \$325,333 \$152,533 \$152,533 \$152,533 \$152,533 \$152,533 \$152,533 \$152,533 \$152,533 \$152,533 \$152,533 \$152,533 \$152,533 \$152,533 \$152,533 \$152,533 \$152,533 <td>MAINLINE:</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>	MAINLINE:							
Type SP Traffic Level D 3 11,484.0 Ton \$85.00 \$0.00 \$976,140 \$976,140 OBG-12 12 70,400.0 \$6, Yd \$15.00 \$0.00 \$1,056,000 \$1,056,000 \$228,800 SHOULDER: 3 3,828.0 Ton \$85.00 \$0.00 \$325,380 \$228,800 SHOULDER: 15 3,828.0 Ton \$85.00 \$0.00 \$325,380 \$325,380 OBG-8 9,5 3,828.0 Ton \$85.00 \$0.00 \$325,380 \$325,380 DESIGN COSTS: 9,5 3,828.0 Ton \$85.00 \$0.00 \$82,523 \$102,00 PE B Sublized LBR 40) 12 3,828.0 Ton \$85.00 \$0.00 \$82,533 \$825,330 DESIGN COSTS: 20 3 7,650.0 Ton \$82.00 \$152,533 \$152,533 Milling 1" Avg. Depth 3 7,650.0 Ton \$85.00 \$0.00 \$82,368 \$50,353 SHOULDER: 1 2 7,650.0 Ton \$85.00 \$0.00 \$83,340 \$264,948 <tr< td=""><td>Type SP Traffic Level D PG76-22</td><td>2</td><td>7.656.0</td><td>Ton</td><td>\$92.00</td><td>\$0.00</td><td>\$704.352</td><td>\$704.352</td></tr<>	Type SP Traffic Level D PG76-22	2	7.656.0	Ton	\$92.00	\$0.00	\$704.352	\$704.352
Type B Stabilized (LBR 40) T2 T0,400.0 Sq. Yd S3.25 S0.00 S228,800 S228,800 SHOULDER: 3.828.0 Ton \$85.00 \$0.00 \$325,380 \$325,380 \$325,380 OBG-8 9.5 46,933.3 Sq. Yd \$14.64 \$50.00 \$687,104 \$687,104 Type B Stabilized (LBR 40) 12 3.828.0 Ton \$885.00 \$6.00 \$525,380 \$325,380 DESIGN COSTS: 2 Subtotal Subtotal Subtotal Subtotal Subtotal Subtotal Subtotal SHOULDER: 14 MAINLINE: 14 Subtotal Subtot	Type SP Traffic Level D		<i>'</i>					
SHOULDER: 3,328.0 Ton \$85.00 \$0.00 \$325,380 \$325,380 OBG-8 9.5 46,933.3 \$0, Yd \$14.64 \$0.00 \$687,104 \$667,104 Type B Stabilized (LBR 40) 12 46,933.3 \$0, Yd \$14.64 \$0.00 \$687,104 \$667,104 DESIGN COSTS: 2 5 5 \$50, Yd \$14.64 \$0.00 \$687,104 \$650,7104 MOT COSTS: 2 5 \$50,000 \$152,533 \$		12.5	70,400.0	Sq. Yd	\$15.00	\$0.00	\$1,056,000	\$1,056,000
Type SP Traffic Level B OBG-8 Type B Stabilized (LBR 40) 1.5 9.5 12 3,828.0 46,933.3 Ton 8,9.7d 6,933.3 S85.00 S,9.7d S,9.7d S14.64 S14.64 S0.00 S0.00 S325,380 S687,104 S457,104 S687,104 DESIGN COSTS: MOT COSTS: CEI COSTS: Image: Cost Simple Stabilized (LBR 40) Image: Cost Simple Stabilized (LBR 40) Image: Cost Simple Stabilized (LBR 40) S152,533	Type B Stabilized (LBR 40)	12	70,400.0	Sq. Yd	\$3.25	\$0.00	\$228,800	\$228,800
Type SP Traffic Level B OBG-8 Type B Stabilized (LBR 40) 1.5 9.5 12 3,828.0 46,933.3 Ton 8,9.7d 6,933.3 S85.00 S,9.7d S,9.7d S14.64 S14.64 S0.00 S0.00 S325,380 S687,104 S457,104 S687,104 DESIGN COSTS: MOT COSTS: CEI COSTS: Image: Cost Simple Stabilized (LBR 40) Image: Cost Simple Stabilized (LBR 40) Image: Cost Simple Stabilized (LBR 40) S152,533								
OBG-8 9.5 46,933.3 Sq. Yd \$14.64 \$0.00 \$687,104 \$687,104 Type B Stabilized (LBR 40) 12 46,933.3 Sq. Yd \$3.25 \$0.00 \$152,533 \$152,533 DESIGN COSTS: MOT COSTS: CEI COSTS: Image: CEI COSTS: Subtotal Subtotal Subtotal Subtotal Subtotal Subtotal Subtotal REHABILITATION IN YEAR: 14 MAINLINE: 14 Subtotal Subtotal \$2000 \$82,368 \$50,885 Type SP Traffic Level D PG76-22 2 7,656.0 Ton \$92.00 \$0.00 \$704,352 \$435,136 SHOULDER: 1 1 46,933.3 \$9, Yd \$2,24 \$0.00 \$105,131 \$64,948 Type SP Traffic Level B 2 46,933.3 \$9, Yd \$2,24 \$0.00 \$105,131 \$64,948 Milling 1" Avg. Depth 1 2 46,933.3 \$9, Yd \$2,24 \$0.00 \$105,131 \$64,948 Stotoal 3 5,104.0 Ton \$85.00 \$0.00 \$433,840	SHOULDER:							
OBG-8 9.5 46,933.3 Sq. Yd \$14.64 \$0.00 \$687,104 \$687,104 Type B Stabilized (LBR 40) 12 46,933.3 Sq. Yd \$3.25 \$0.00 \$152,533 \$152,533 DESIGN COSTS: MOT COSTS: CEI COSTS: Image: CEI COSTS: Subtotal Subtotal Subtotal Subtotal Subtotal Subtotal Subtotal REHABILITATION IN YEAR: 14 MAINLINE: 14 Subtotal Subtotal \$2000 \$82,368 \$50,885 Type SP Traffic Level D PG76-22 2 7,656.0 Ton \$92.00 \$0.00 \$704,352 \$435,136 SHOULDER: 1 1 46,933.3 \$9, Yd \$2,24 \$0.00 \$105,131 \$64,948 Type SP Traffic Level B 2 46,933.3 \$9, Yd \$2,24 \$0.00 \$105,131 \$64,948 Milling 1" Avg. Depth 1 2 46,933.3 \$9, Yd \$2,24 \$0.00 \$105,131 \$64,948 Stotoal 3 5,104.0 Ton \$85.00 \$0.00 \$433,840								
OBG-8 9.5 46,933.3 Sq. Yd \$14.64 \$0.00 \$687,104 \$687,104 Type B Stabilized (LBR 40) 12 46,933.3 Sq. Yd \$3.25 \$0.00 \$152,533 \$152,533 DESIGN COSTS: MOT COSTS: CEI COSTS: Image: CEI COSTS: Subtotal Subtotal Subtotal Subtotal Subtotal Subtotal Subtotal REHABILITATION IN YEAR: 14 MAINLINE: 14 Subtotal Subtotal \$2000 \$82,368 \$50,885 Type SP Traffic Level D PG76-22 2 7,656.0 Ton \$92.00 \$0.00 \$704,352 \$435,136 SHOULDER: 1 1 46,933.3 \$9, Yd \$2,24 \$0.00 \$105,131 \$64,948 Type SP Traffic Level B 2 46,933.3 \$9, Yd \$2,24 \$0.00 \$105,131 \$64,948 Milling 1" Avg. Depth 1 2 46,933.3 \$9, Yd \$2,24 \$0.00 \$105,131 \$64,948 Stotoal 3 5,104.0 Ton \$85.00 \$0.00 \$433,840	Type SP Traffic Level B	1.5	3,828.0	Ton	\$85.00	\$0.00	\$325.380	\$325.380
DESIGN COSTS: Subtotal MOT COSTS: Subtotal CEI COSTS: 14 MAINLINE: 14 MAINLINE: 3 Milling 3" Avg. Depth 3 Type SP Traffic Level D PG76-22 2 7,656.0 Ton SHOULDER: 1 Milling 1" Avg. Depth 1 Type SP Traffic Level B 2 SHOULDER: 1 Milling 1" Avg. Depth 1 A6,933.3 Sq. Yd \$2.24 SHOULDER: 5,104.0 Type SP Traffic Level B 2 SHOULDER: 2 Milling 1" Avg. Depth 1 SHOULDER: 3 Milling 1" Avg. Depth 1 SHOULDER: 2 Milling 1" Avg. Depth 1 Should 5,104.0 Ton \$85.00 \$0.00 \$433,840 \$268,018 DESIGN COSTS: Subtotal								
MOT COSTS: Subtotal Subtotal CEI COSTS: 14 MAINLINE: 14 MAINLINE: 2 Milling 3" Avg. Depth 3 Type SP Traffic Level D PG76-22 2 7,656.0 Ton SPOULDER: 2 Milling 1" Avg. Depth 1 Milling 1" Avg. Depth 1 1 46,933.3 Sq. Yd \$2.24 Stotal \$402,028 Subtotal 5,104.0 Type SP Traffic Level B 2 Setter 2 Milling 1" Avg. Depth 1 1 46,933.3 Sq. Yd \$2.24 Stoto \$402,028 Subtotal \$2.24 Stoto \$433,840 \$268,018 Subtotal \$40,933.3 Sq. Yd \$2.24 Stoto \$433,840 \$268,018 \$268,018 Subtotal \$2.104.0 Ton \$85.000 \$433,840	Type B Stabilized (LBR 40)	12	46,933.3	Sq. Yd	\$3.25	\$0.00	\$152,533	\$152,533
CEI COSTS: Subtotal REHABILITATION IN YEAR: 14 MAINLINE: 3 Milling 3" Avg. Depth 3 Type SP Traffic Level D PG76-22 2 PSP Traffic Level D PG76-22 2 Type SP Traffic Level D PG76-22 2 SHOULDER: 1 Milling 1" Avg. Depth 1 SHOULDER: 1 Milling 1" Avg. Depth 1 SHOULDER: 1 Milling 1" Avg. Depth 1 DESIGN COSTS: Subtotal MOT COSTS: Subtotal	DESIGN COSTS:			Subtotal				
REHABILITATION IN YEAR: 14 MAINLINE: 3 70,400.0 Sq. Yd \$1.17 \$0.00 \$82,368 \$50,885 Type SP Traffic Level D PG76-22 2 7,656.0 Ton \$92.00 \$0.00 \$704,352 \$435,136 Type SP Traffic Level D 2 7,656.0 Ton \$92.00 \$0.00 \$650,760 \$402,028 SHOULDER: 2 5,106.0 Ton \$85.00 \$0.00 \$105,131 \$64,948 Type SP Traffic Level B 2 5,104.0 Ton \$85.00 \$0.00 \$433,840 \$268,018 DESIGN COSTS: Subtotal Subtotal Subtotal Subtotal Subtotal	MOT COSTS:			Subtotal				
MAINLINE: 3 70,400.0 Sq. Yd \$1.17 \$0.00 \$82,368 \$50,885 Type SP Traffic Level D PG76-22 2 7,656.0 Ton \$92.00 \$0.00 \$704,352 \$435,136 Type SP Traffic Level D 2 7,656.0 Ton \$85.00 \$0.00 \$650,760 \$402,028 SHOULDER: 46,933.3 Sq. Yd \$2.24 \$0.00 \$105,131 \$64,948 Type SP Traffic Level B 2 5,104.0 Ton \$85.00 \$0.00 \$433,840 \$268,018 DESIGN COSTS: MOT COSTS: L Subtotal Subtotal Subtotal Subtotal	CEI COSTS:			Subtotal				
MAINLINE: Milling 3" Avg. Depth 3 70,400.0 Sq. Yd \$1.17 \$0.00 \$82,368 \$50,885 Type SP Traffic Level D PG76-22 2 7,656.0 Ton \$92.00 \$0.00 \$704,352 \$435,136 Type SP Traffic Level D 2 7,656.0 Ton \$85.00 \$0.00 \$650,760 \$402,028 SHOULDER: 46,933.3 Sq. Yd \$2.24 \$0.00 \$105,131 \$64,948 Type SP Traffic Level B 2 5,104.0 Ton \$85.00 \$0.00 \$433,840 \$268,018 DESIGN COSTS: MOT COSTS: Subtotal Subtotal Subtotal Subtotal Subtotal	REHABILITATION IN YEAR:	14	1					
Type SP Traffic Level D PG76-22 2 7,656.0 Ton \$92.00 \$0.00 \$704,352 \$435,136 Type SP Traffic Level D 2 7,656.0 Ton \$85.00 \$0.00 \$650,760 \$402,028 SHOULDER: 1 46,933.3 Sq. Yd \$2.24 \$0.00 \$105,131 \$64,948 Type SP Traffic Level B 2 5,104.0 Ton \$85.00 \$0.00 \$433,840 \$268,018 DESIGN COSTS: MOT COSTS: Subtotal Subtotal Subtotal Subtotal	MAINLINE:		-					
Type SP Traffic Level D 2 7,656.0 Ton \$85.00 \$0.00 \$650,760 \$402,028 SHOULDER: 46,933.3 Sq. Yd \$2.24 \$0.00 \$105,131 \$64,948 Type SP Traffic Level B 2 46,933.3 Sq. Yd \$2.24 \$0.00 \$105,131 \$64,948 DESIGN COSTS: Subtotal Subtotal Subtotal Subtotal Subtotal Subtotal Subtotal Subtotal Subtotal Subtotal Subtotal Subtotal Subtotal Subtotal Subtotal Subtotal Subtotal Subtotal Subtotal Subtotal Subtotal Subtotal Subtotal Subtotal Subtotal Subtotal Subtotal Subtotal Subtotal Subtotal Subtotal Subtotal Subtotal Subtotal		3	70,400.0	Sq. Yd	\$1.17	\$0.00	\$82,368	\$50,885
SHOULDER: 1 46,933.3 Sq. Yd \$2.24 \$0.00 \$105,131 \$64,948 Type SP Traffic Level B 2 5,104.0 Ton \$85.00 \$0.00 \$433,840 \$268,018 DESIGN COSTS: Subtotal Subtotal Subtotal Subtotal Subtotal								
Milling 1" Avg. Depth 1 46,933.3 Sq. Yd \$2.24 \$0.00 \$105,131 \$64,948 Type SP Traffic Level B 2 5,104.0 Ton \$85.00 \$0.00 \$433,840 \$268,018 DESIGN COSTS: Subtotal Subtotal Subtotal	Type SP Traffic Level D	2	7,656.0	Ton	\$85.00	\$0.00	\$650,760	\$402,028
Milling 1" Avg. Depth 1 46,933.3 Sq. Yd \$2.24 \$0.00 \$105,131 \$64,948 Type SP Traffic Level B 2 5,104.0 Ton \$85.00 \$0.00 \$433,840 \$268,018 DESIGN COSTS: Subtotal Subtotal Subtotal	SHOULDER:							
Type SP Traffic Level B 2 5,104.0 Ton \$85.00 \$0.00 \$433,840 \$268,018 DESIGN COSTS: Subtotal		1	46,933.3	Sq. Yd	\$2.24	\$0.00	\$105,131	\$64,948
MOT COSTS: Subtotal								
MOT COSTS: Subtotal								
MOT COSTS: Subtotal	DESIGN COSTS.			0.1.				
A DIDIDIZI	CEI COSTS:			Subtotal				

LIFE CYCLE COST ANALYSIS ASPHALT CONCRETE PAVEMENT DESIGN (FLEXIBLE PAVEMENT) Financial Project ID:432100-1-22-01, SR No.-SR 400, County:Volusia



Project Length: 10 Miles, Roadway ID: 79110000

Begining MP: , Ending MP:

Definitions:	
Length of Section:	5280
Passing Lane Width:	12
Travel Lane Width:	12
Inside Shoulder Width:	18
Outside Shoulder Width:	22
Total Pavement Area:	633,600
Total Shoulder Area:	422,400

Ft Ft Ft

Ft Ft

Sq. Ft

Sq. Ft

Analysis Period:
Discount Rate:
Initial Year of Construction:
No. of Passing Lanes:
No. of Travel Lanes:
No. of Travel Directions:

40
3.5
2020
5
2

CONSTRUCTION ITEMS	ТНК.	QTY.	UNIT	UNIT PRICE	ST DEV	COST	PRESENT WORTH
REHABILITATION IN YEAR: MAINLINE:	28]					
Milling 3" Avg. Depth	3	70,400.0	Sq. Yd	\$1.17	\$0.00	\$82,368	\$31,436
Type SP Traffic Level D PG76-22	2	7,656.0	Ton	\$92.00	\$0.00	\$704,352	\$268,819
Type SP Traffic Level D	2	7,656.0	Ton	\$85.00	\$0.00	\$650,760	\$248,365
						. ,	
SHOULDER:							
Milling 1" Avg. Depth	1	46,933.3	Sq. Yd	\$2.24	\$0.00	\$105,131	\$40,124
Type SP Traffic Level B	2	5,104.0	Ton	\$85.00	\$0.00	\$433,840	\$165,577
DESIGN COSTS:			Subtotal				
MOT COSTS:			Subtotal				
CEI COSTS:			Subtotal				
REHABILITATION IN YEAR: MAINLINE:	42]					
SHOULDER:							
DESIGN COSTS:			Subtatal				
MOT COSTS:			Subtotal				
CEI COSTS:			Subtotal Subtotal				
			Subtotal				
REHABILITATION IN YEAR:]					
	TOTAL INITIA	L CONSTRU	UCTION CO	ST (YEAR 2020):			\$4,130,309
	TOTAL PRES	ENT WORT	H REHABII	LITATION COST:			\$1,975,336
PART OF TRANSPORT	ΤΟΤΑ	L PRESENT	WORTH SA	ALVAGE VALUE:			\$71,314
				ESENT WORTH:			\$6,034,332



FLORIDA DEPARTMENT OF TRANSPORTATION PAVEMENT TYPE SELECTION ECONOMIC ANALYSIS COST PER MILE

Analysis Period:	40	Years	Disco	ount Rate:	3.5%	
PCC PAVEME	<u>NT</u>			D (D		
	.	<u>Cost</u>	.4.	$\underline{\mathbf{P}/\mathbf{F}}$		RESENT WORTI
	Initial	\$6,413,778	-	1.00000	= -	\$6,413,778
23	Year	\$590,219	*	0.45329	=	\$267,538
33	Year	\$790,468	-	0.32134	=	\$254,011
40	Year		*		= .	
	Year	Т	OTAL	AGENCY COSTS	=	\$6,935,327
				USER COSTS	=	
				PW of Last Rehab	-	
	Ī	Remaining Service I	<u>life</u>	<u>at Year 40</u>		
SALVAGE	E VALUE	0 / 7	*	\$199,650	=	\$0
	TOTAL	L PRESENT WORT	TH LIF	E-CYCLE COSTS	=	\$6,935,327
	т					
AC PAVEMEN	<u>1</u>	Cost		<u>P / F</u>	<u>P</u>	RESENT WORT
	Initial	\$4,130,309	*	1.00000	=	\$4,130,309
14	Year	\$1,976,451	*	0.61778	=	\$1,221,015
28	Year	\$1,976,451	*	0.38165	=	\$754,321
42	Year		*		=	
	Year		-		-	
		Т	OTAL	AGENCY COSTS	= _	\$6,105,646
				USER COSTS	= _	
				PW of Last Rehab		
		<u>Remaining Service I</u>		<u>at Year 40</u>		
SALVAGE		2 / 14	-	\$499,197	= -	\$71,314
	TOTAL	L PRESENT WORT	TH LIF	E-CYCLE COSTS	=	\$6,034,332
COST COMPA	RISON					
		L PRESENT WORT	TH LIF	E-CYCLE COSTS	=	\$900,995
		AVERAGE TO	TAL PI	RESENT WORTH	= -	\$6,484,829
J	PERCENT DI	FFERENCE IN TO			= -	13.9%
					-	
	DIFF	ERENCE IN ESTIN	MATED	INITIAL COSTS	=	\$2,283,469
PE	RCENT DIFF	ERENCE IN ESTIN	ИАТЕД	INITIAL COSTS	= -	55.3%
					-	*
TOTAL PRES	ENT WORTH	I COST OF REHAI	B FOR I	PCC PAVEMENT	=	\$521,549
					-	
TOTAL PRE	SENT WORT	TH COST OF REHA	AB FOR	AC PAVEMENT	=	\$1,975,336

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Florida Department of Transportation Item Average Unit Cost From 2013/03/01 to 2014/02/28 Page:

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Contract Type: CC AREAS: 06 Displaying: VALID ITEMS WITH HITS From: 0102 1 To: 9999999

Item	No. of Conts	Weighted Average	Total Amount	Total Quantity	Unit Meas	Obs?	Description
0102 1	3	\$230.48	\$299,618.40	1,300.000	DA	N	MAINTENANCE OF TRAFFIC
0102 14	1	\$50.00	\$3,200.00	64.000	MH	Ν	TRAFFIC CONTROL OFFICER
0102 60	2	\$.31	\$9,310.80	29,712.000	ED	Ν	WORK ZONE SIGN
0102 74 1	2	\$.19	\$11,907.22	63,830.000	ED	Ν	TEMP BARR-TYPS I,II,DI,VP, DRUM, LC
0102 76	2	\$7.46	\$4,590.00	615.000	ED	Ν	ARROW BOARD /ADVANCE WARNING ARROW PANEL
0102 77	2	\$.33	\$4,652.05	14,069.000	ED	Ν	HIGH INTENSITY FLASH LI, TEMP, TYP B
0102 78	1	\$2.15	\$6,450.00	3,000.000	EA	Ν	TEMPORARY RETROREFLECTIVE PAVT MARKER
0102 99	2	\$13.67	\$17,044.00	1,247.000	ED	Ν	PORTABLE CHANGEABLE MESSAGE SIGN, TEMP
0102150 1	2	\$8.24	\$10,130.00	1,230.000	ED	Ν	PORTABLE REGULATORY, SIGN
0102150 2	2	\$8.24	\$10,130.00	1,230.000	ED	Ν	RADAR SPEED DISPLAY UNIT
0104 10 3	2	\$1.00	\$20,359.75	20,387.000	LF	Ν	SEDIMENT BARRIER
0104 18	2	\$77.64	\$3,261.00	42.000	EA	Ν	INLET PROTECTION SYSTEM
0107 1	1	\$15.00	\$16,800.00	1,120.000	AC	Ν	LITTER REMOVAL
0107 2	1	\$20.00	\$22,400.00	1,120.000	AC	Ν	MOWING
0110 1 1	2	\$9,218.67	\$27,656.00	3.000	AC	Ν	CLEARING & GRUBBING
0110 4	1	\$19.00	\$6,099.00	321.000	SY	Ν	REMOVAL OF EXISTING CONCRETE PAVEMENT
0110 7 1	1	\$110.00	\$1,320.00	12.000	EA	Ν	MAILBOX, F&I SINGLE
0120 2 2	1	\$12.82	\$179,736.40	14,020.000	CY	Ν	BORROW EXCAVATION, TRUCK MEASURE
0120 71	1	\$55,562.86	\$55,562.86	1.000	LS	Ν	REGULAR EXCAVATION (3-R PROJECTS ONLY)
0162 1 11	1	\$.27	\$65,870.82	243,966.000	SY	Ν	PREPARED SOIL LAYER, FINISH SOIL, 6"
0285709	1	\$15.38	\$207,630.00	13,500.000	SY	Ν	OPTIONAL BASE, BASE GROUP 09
0327 70 4	1	\$1.17	\$62,736.57	53,621.000	SY	N	MILLING EXIST ASPH PAVT, 3" AVG DEPTH
0327 70 6	1	\$1.90	\$84,080.70	44,253.000	SY	Ν	MILLING EXIST ASPH PAVT,1 1/2" AVG DEPTH
0327 70 11	1	\$.68	\$288,856.52	424,789.000	SY	Ν	MILLING EXIST ASPH PAVT,2 1/4" AVG DEPTH
0334 1 23	1	\$85.98	\$3,778,906.98	43,951.000	TN	Ν	SUPERPAVE ASPH CONC, TRAF C, PG76-22,PMA
0337 7 22	1	\$113.00	\$1,856,861.20	16,432.400	TN	Ν	ASPH CONC FC, INC BIT, FC-5, PG76-22, PMA
0337 7 43	1	\$95.20	\$788,941.44	8,287.200	TN	Ν	ASPH CONC FC, TRAFFIC C, FC-12.5, PG 76-22
0339 1	2	\$132.22	\$6,240.92	47.200	TN	Ν	MISCELLANEOUS ASPHALT PAVEMENT
0400 32	1	\$14,800.00	\$128,760.00	8.700	CY	Ν	CONCRETE FOR JOINT REPAIR
0401 70 1	1	\$100.00	\$2,000.00	20.000	CF	N	RESTORE SPALLED AREAS, EPOXY
0413151	1	\$100.00	\$756,700.00	7,567.000	GA	Ν	METHACRYLATE MONOMER
0413154	1	\$.30	\$226,907.40	756,358.000	SF	Ν	CLEAN & SEAL CONC- PENETR OR METHACR
0415 1 4	1	\$2.60	\$5,150.60	1,981.000	LB	N	REINF STEEL- SUPERSTRUCTURE
0425 6	1	\$390.00	\$390.00	1.000	EA	Ν	VALVE BOXES, ADJUST
0430174118	1	\$51.42	\$31,674.72	616.000	\mathbf{LF}	Ν	PIPE CULV, OPT MATL, ROUND,18"SD
0430174124	1	\$55.60	\$16,902.40	304.000	\mathbf{LF}	Ν	PIPE CULV, OPT MATL, ROUND,24"SD
0430174130	1	\$65.29	\$6,267.84	96.000	\mathbf{LF}	Ν	PIPE CULV, OPT MATL, ROUND,30"SD
0430174215	1	\$49.27	\$3,941.60	80.000	\mathbf{LF}	Ν	PIPE CULV, OPT MATL, OTHER, 15"SD
0430174218	1	\$51.55	\$1,649.60	32.000	\mathbf{LF}	Ν	PIPE CULV, OPT MATL, OTHER, 18"SD
0430984125	1	\$894.00	\$31,290.00	35.000	EA	Ν	MITERED END SECT, OPTIONAL RD, 18" SD

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Florida Department of Transportation Item Average Unit Cost From 2013/03/01 to 2014/02/28

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Contract Type: CC STATEWIDE Displaying: VALID ITEMS WITH HITS From: 0102 1 To: 9999999

	No. of	Weighted	Total	Total	Unit		
Item	Conts	Average	Amount	Quantity	Meas	Obs?	Description
0125 1	5	\$12.25	\$189,709.27	15,484.000	CY	N	EXCAVATION FOR STRUCTURES
0142 70	2	\$8.30	\$254,775.45	30,698.900	CY	N	FILL SAND
0145 1	1	\$2.80	\$34,034.00	12,155.000	SF	N	GEOSYNTHETIC REINFORCED SOIL SLOPE
0145 2	5	\$4.13	\$128,153.92	31,015.000	SY	N	GEOSYNTHETIC REINF FND OVER SOFT SOIL
0145 71	1	\$3.30	\$20,697.60	6,272.000	SY	N	REINFORCEMENT GRID FOR SOIL STABILIZAT
0160 4	75	\$3.06	\$6,414,990.51	2,099,340.600	SY	N	TYPE B STABILIZATION
0162 1 11	47	\$.80	\$1,311,291.19	1,639,557.500	SY	N	PREPARED SOIL LAYER, FINISH SOIL, 6"
0162 1 12	3	\$6.73	\$192,723.16	28,643.000	SY	N	PREPARED SOIL LAYER, FINISH SOIL, 12"
0162 1 33	2	\$6.47	\$19,914.72	3,078.000	SY	N	PREPARED SOIL LAYER, BLANKET, SPECIAL
0210 1 1	3	\$.84	\$15,497.22	18,428.000	SY	N	REWORKING LIMEROCK BASE, 6"
0210 1 8	1	\$5.25	\$7,612.50	1,450.000	SY	Ν	REWORKING LIMEROCK BASE, 4"
0210 2	2	\$27.26	\$13,902.33	510.000	CY	N	LIMEROCK-NEW MATERIAL FOR REWORKING BASE
0285701	50	\$9.42	\$2,282,760.45	242,216.300	SY	N	OPTIONAL BASE, BASE GROUP 01
0285702	6	\$8.78	\$985,403.12	112,285.600	SY	N	OPTIONAL BASE, BASE GROUP 02
0285703	4	\$20.07	\$424,418.92	21,145.000	SY	N	OPTIONAL BASE, BASE GROUP 03
0285704	16	\$12.37	\$1,641,097.48	132,698.100	SY	N	OPTIONAL BASE, BASE GROUP 04
0285705	5	\$9.81	\$215,501.27	21,972.500	SY	N	OPTIONAL BASE, BASE GROUP 05
0285706	23	\$16.77	\$2,573,079.32	153,469.000	SY	Ν	OPTIONAL BASE, BASE GROUP 06
0285707	6	\$16.12	\$571,196.20	35,437.000	SY	Ν	OPTIONAL BASE, BASE GROUP 07
0285708	3	\$14.64	\$94,931.10	6,484.000	SY	N	OPTIONAL BASE, BASE GROUP 08
0285709	43	\$19.73	\$6,878,342.28	348,549.700	SY	N	OPTIONAL BASE, BASE GROUP 09
0285710	11	\$11.93	\$2,185,574.32	183,246.000	SY	Ν	OPTIONAL BASE, BASE GROUP 10
0285711	14	\$12.95	\$7,824,233.51	604,391.000	SY	Ν	OPTIONAL BASE, BASE GROUP 11
0285712	7	\$21.90	\$431,998.20	19,729.000	SY	Ν	OPTIONAL BASE, BASE GROUP 12
0285713	7	\$39.77	\$1,296,066.58	32,589.000	SY	Ν	OPTIONAL BASE, BASE GROUP 13
0285715	14	\$45.07	\$3,044,657.79	67,555.900	SY	Ν	OPTIONAL BASE, BASE GROUP 15
0286 1	26	\$13.26	\$1,031,742.02	77,795.600	SY	Ν	TURNOUT CONSTRUCTION
0286 2	2	\$151.17	\$48,737.50	322.400	TN	Ν	TURNOUT CONSTRUCTION-ASPHALT
0287 1	1	\$160.00	\$929,600.00	5,810.000	CY	Ν	ASPHALT TREATED PERMEABLE BASE
0288001	1	\$800.00	\$357,600.00	447.000	CY	Ν	CEMENT TREATED PERMEABLE BASE
0327 70 1	49	\$2.24	\$2,747,112.05	1,226,732.900	SY	N	MILLING EXIST ASPH PAVT, 1" AVG DEPTH
0327 70 2	9	\$2.10	\$965,982.91	459,819.000	SY	Ν	MILLING EXIST ASPH PAVT,3 1/2" AVG DEPTH
0327 70 4	20	\$1.99	\$2,218,028.11	1,113,562.100	SY	Ν	MILLING EXIST ASPH PAVT, 3" AVG DEPTH
0327 70 5	34	\$2.31	\$2,885,791.32	1,248,080.000	SY	Ν	MILLING EXIST ASPH PAVT, 2" AVG DEPTH
0327 70 6	59	\$1.49	\$3,636,071.26	2,436,213.940	SY	Ν	MILLING EXIST ASPH PAVT,1 1/2" AVG DEPTH
0327 70 7	4	\$3.93	\$499,059.98	126,869.000	SY	N	MILLING EXIST ASPH PAVT, 4" AVG DEPTH
0327 70 8	17	\$1.74	\$1,462,074.05	838,132.000	SY	Ν	MILLING EXIST ASPH PAVT,2 1/2" AVG DEPTH
0327 70 10	1	\$8.00	\$15,888.00	1,986.000	SY	Ν	MILLING EXIST ASPH PAVT, 5" AVG DEPTH
0327 70 11	14	\$1.57	\$2,308,746.06	1,472,431.000	SY	N	MILLING EXIST ASPH PAVT,2 1/4" AVG DEPTH
0327 70 12	6	\$1.75	\$161,377.52	92,371.000	SY	N	MILLING EXIST ASPH PAVT,1 1/4" AVG DEPTH

CESPO05 03/24/2014-07.00.01

Florida Department of Transportation Item Average Unit Cost From 2013/03/01 to 2014/02/28

Contract Type: CC STATEWIDE Displaying: VALID ITEMS WITH HITS From: 0102 1 To: 9999999

	No. of	Weighted	Total	Total	Unit		
Item	Conts	Average	Amount	Quantity	Meas	Obs?	Description
0443 70 6	2	\$170.95	\$77,099.50	451.000	LF	Ν	FRENCH DRAIN, 36"
0444 70 11	1	\$151.17	\$3,023.40	20.000	LF	Ν	DEEP WELL- OPEN HOLE, 24"
0444 71 11	1	\$251.95	\$22,675.50	90.000	LF	N	DEEP WELL CASING, 24"
0446 1 1	2	\$26.72	\$213,892.08	8,004.000	LF	N	EDGEDRAIN DRAINCRETE, STANDARD
0446 71 1	4	\$30.67	\$56,408.70	1,839.000	LF	N	EDGEDRAIN OUTLET PIPE, 4"
0448 73	1	\$873,909.95	\$873,909.95	1.000	LS	N	PUMPING STATION- DRAINAGE
0450 1251	1	\$175.00	\$253,750.00	1,450.000	LF	N	PREST BEAMS, INVERTED T FROM FIB, 26.5"
0450 2 36	5	\$237.53	\$3,113,565.00	13,108.000	LF	N	PREST BEAMS: FLORIDA-I BEAM 36"
0450 2 45	5	\$192.11	\$1,749,757.99	9,108.000	LF	N	PREST BEAMS: FLORIDA-I BEAM 45"
0450 2 54	1	\$210.00	\$1,180,830.00	5,623.000	LF	N	PREST BEAMS: FLORIDA-I BEAM 54"
0450 2 63	1	\$215.00	\$365,930.00	1,702.000	LF	Ν	PREST BEAMS: FLORIDA-I BEAM 63"
0450 2 84	1	\$250.00	\$332,250.00	1,329.000	LF	N	PREST BEAMS: FLORIDA-I BEAM 84"
0450 3 15	1	\$160.00	\$50,240.00	314.000	LF	N	PRESTRESSED SLAB UNITS,48" X 15"
0450 3 25	1	\$160.00	\$150,400.00	940.000	LF	Ν	PRESTRESSED SLAB UNITS,60" X 15"
0450 3 95	1	\$160.00	\$50,240.00	314.000	LF	Ν	PRESTRESSED SLAB UNITS,VAR WI 30-47", 15
0450 82	1	\$400.00	\$90,000.00	225.000	LF	Ν	BEAM REPAIR
0450 83 1	2	\$597.84	\$57,392.40	96.000	EA	Ν	BEAM REPAIR, STRAND SPLICES
0450 88 20	1	\$71.00	\$127,161.00	1,791.000	SF	N	PRESTR SLAB UNITS TRANSV POST TENS, 20"
0455 2	1	\$1.00	\$4,230.00	4,230.000	LF	N	TREATED TIMBER PILING
0455 14 3	2	\$89.64	\$520,335.80	5,805.000	LF	Ν	CONC SHEET PILING, 10"X30"
0455 14 4	1	\$379.05	\$191,041.20	504.000	LF	N	CONC SHEET PILING, 12"X30"
0455 18	7	\$21,232.12	\$148,624.82	7.000	LS	N	PROTECTION OF EXISTING STRUCTURES
0455 34 2	2	\$72.14	\$1,206,550.00	16,726.000	LF	Ν	PRESTRESSED CONCRETE PILING, 14" SQ.
0455 34 3	6	\$78.40	\$2,009,045.75	25,624.000	LF	N	PRESTRESSED CONCRETE PILING, 18" SQ
0455 34 5	9	\$102.39	\$4,084,021.40	39,887.000	LF	N	PRESTRESSED CONCRETE PILING, 24" SQ
0455 35 6	1	\$80.00	\$9,680.00	121.000	LF	N	STEEL PILING, HP 14 X 89
0455 35 8	1	\$152.45	\$134,156.00	880.000	LF	N	STEEL PILING, HP 14 X 117
0455 35 21	1	\$166.92	\$238,862.52	1,431.000	LF	N	STEEL PILING, 20" DIA. PIPE
0455 35 22	3	\$117.76	\$627,170.00	5,326.000	LF	N	STEEL PILING, 24" DIA. PIPE
0455 81101	1	\$5,000.00	\$20,000.00	4.000	EA	N	CATHODIC PROT,F&I,PILE,ZINC ANODE ASSEM
0455 81105	1	\$6,875.00	\$385,000.00	56.000	EA	N	CATHODIC PROT, F&I, PIER, TITANIUM ANODE
0455 87	3	\$368.90	\$29,881.30	81.000	EA	N	ANCHOR BAR, STEEL
0455 88 5	1	\$335.72	\$692,926.08	2,064.000	LF	N	DRILLED SHAFT, 48" DIA
0455 88 6	1	\$150.00	\$167,700.00	1,118.000	LF	N	DRILLED SHAFT, 60" DIA
0455101 1	1	\$215,459.75	\$215,459.75	1.000	EA	N	TEST LOAD, OSTERBERG CELL, < FIVE CELLS
0455107 5	1	\$169.99	\$34,677.96	204.000	LF	N	DRILLED SHAFT CASING, 48" DIA
0455107 6	1	\$231.00	\$160,776.00	696.000	$_{ m LF}$	N	DRILLED SHAFT CASING, 60" DIA
0455111	2	\$75.24	\$265,580.00	3,530.000	$_{ m LF}$	N	CORE-PILOT HOLE, DRILLED SHAFT EXCAV
0455120 7	3	\$677.78	\$67,100.00	99.000	EA	N	PILE POINT PROTECTION, 24" ROUND
0455122 5	1	\$199.17	\$379,219.68	1,904.000	LF	N	UNCLASSIFIED SHAFT EXCAVATION, 48" DIA

APPENDIX F

PAVEMENT PERFORMANCE DATA

Rehabilitation Age by Year

For Volusia County 19MAR2014 Other Conditions: Pavement= Asphalt

Year Rehabilitated	Lane Miles Rehabilitated	Average Rehabilitation Age	Standard Deviation
2006	165.9	12.8	6.2
2007	83.5	14.0	5.2
2008	65.0	13.7	5.6
2009	37.7	16.7	8.3
2010	73.1	17.5	4.5
2011	164.2	16.7	4.8
2012	26.8	14.7	2.4
2013	17.0	11.4	2.4

Rehabilitation Age by Year For Seminole County

For Seminole County 27FEB2014 Other Conditions: Pavement= Asphalt

Year Rehabilitated	Lane Miles Rehabilitated	Average Rehabilitation Age	Standard Deviation
2006	17.1	20.0	5.3
2007	29.5	12.8	3.2
2008	42.3	20.1	9.2
2009	15.1	13.9	6.6
2010	43.6	16.0	1.9
2011	33.2	22.1	11.3
2012	6.5	15.0	0.0
2013	39.9	14.7	5.3

Deficient Rehabilitation age by Year For Hillsborough County

Other Conditions: Pavement= Concrete

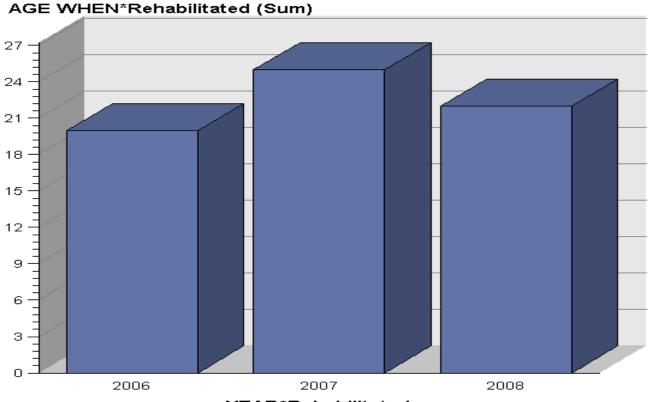
Surface Type in (CONC)

Year Rehabilitated	Lane Miles Rehabilitated	Average Rehabilitation Age	Standard Deviation
2006	10.8	20	0
2007	26.7	25	0
2008	9.3	22	0

Deficient Rehabilitation age by Year For Hillsborough County 13JUN2012

For Hillsborough County Other Conditions: Pavement= Concrete

Surface Type in (CONC)



YEAR*Rehabilitated

13JUN2012

APPENDIX G

QUALITY CONTROL CHECKLIST

PAVEMENT TYPE SELECTION

QUALITY CONTROL CHECKLIST

	Satisfactory
	Yes / No
Project Description	yes
Financial Project ID / Annual Report	yes
State Road No	yes
County	yes
Project Length	yes
Transportation System	yes
Flexible Pavement Design	
ESAL	yes
Level of Reliability	yes
Initial Design Period	yes
Structural Number	yes
Friction Course	yes
Structural Thickness	yes
Base Thickness	yes
Number of Through	yes
Lanes	
Lane Width	yes
Shoulder Width	yes

Rigid Pavement Design

Rigid Pavement Design	
ESAL	yes
Level of Reliability	yes
Initial Design Period	yes
Thickness	yes

yes

Base Thickness	405
Base Type	yes
Number of Through	yes
Lanes	1
Lane Width	yes
Shoulder Width	yes
Design Method (AASHTO 1993 or MEPDG)	yes

PROJECT MILE ESTIMATES

Initial

Mainline Quantities	yes
Shoulder Quantities	yes
Unit Prices Reasonable	yes

Rehabilitation

Mair	line Quantities
Shou	ulder Quantities
Unit	Prices Reasonable

meli 0

Reviewer Signature

Date